

GENERAL

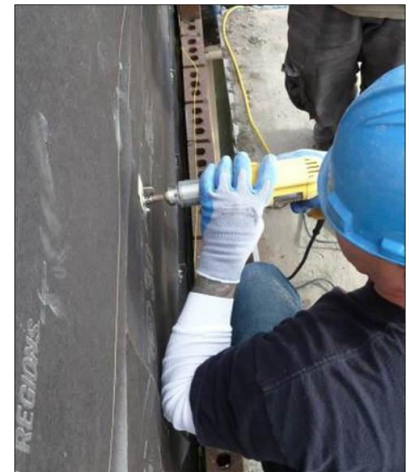
Brick tie requirements are outlined in *CSA A370-04 Connectors for Masonry*. The older kinds of ties, such as strip ties and Z ties as are seldom used in modern commercial construction, can not be used in higher seismic zones, and are now referred to as “Prescriptive Ties”. The newer, 2-piece, adjustable, engineered ties that are now in common use are now simply referred to as “Ties”. *CSA A370-04* contains strict design requirements for strength, deflection and free play. Ties are designed to resist the lateral wind and seismic loads provided for specific locations by the B.C. *Building Code (BCBC)*. Factored Tie Capacities are normally provided by test data from the manufacturers.

Corrosion resistance is a key requirement for ties which are required to secure masonry claddings over their long expected life. The section below updates the corrosion requirements first introduced in the 1994 standard, which included the use of stainless steel ties in higher masonry walls in regions of the country which experience high wind-driven rain conditions.

STAINLESS STEEL TIES

The 2006 BCBC references the 2004 edition of *CSA A370*, "Connectors for Masonry". The standard requires stainless steel ties for masonry over 13 m high (formerly “buildings” over 11 m) for areas subject to high wind-driven rain such as coastal B.C. Hot dipped galvanized coatings are acceptable corrosion protection for walls 13 m or lower in coastal B.C., and for all wall heights in the drier areas of B.C. The standard provides rain data for locations across Canada in Annex E, in terms of Annual Driving Rain Indices (aDRI).

To take full advantage of the very long service life offered by masonry veneers, stainless steel ties may also be specified for lower walls on institutional or monumental buildings. It may also be simpler to specify stainless steel ties for the lower walls of buildings using them on higher levels of the structure. In any case, the impact on total wall cost is



Ties must be labeled:

Tie packaging or pieces must be labeled, including corrosion protection type.

aDRI Values in BC:

Abbotsford	4.13*
Cranbrook	0.73
Kamloops	0.61
Port Alberni	2.94*
Prince George	1.15
Prince Rupert	9.37*
Vancouver	3.72*

relatively minor. Stainless steel ties are readily available, and have been used on many projects on the west coast for over a decade.

TIE DESIGN for SEISMIC LOADS

Earthquake lateral loads on brick ties are determined by the formula for elements and components of buildings and their connections from the BCBC (clause 4.1.8.17):

$$V_p = 0.3 F_a S_a(0.2) I_E S_p W_p$$

Where: V_p = lateral force

F_a = acceleration based site factor (soil type)

$S_a(0.2)$ = spectral response acceleration (seismic factor)

I_E = building importance factor:

1.0; except 1.3 for schools, etc.; 1.5 for post-disaster

$S_p = C_p A_r A_x / R_p$ (0.7 < S_p < 4.0)

C_p = component factor: 1.0 for ties

A_r = component force amplification factor: 1.0 for ties

A_x = height factor: 3.0, worst case at top of wall for ties

R_p = component response modification factor:

1.5 for ties, 1.0 for fasteners

W_p = weight of component: take as 1.8 kN/m²

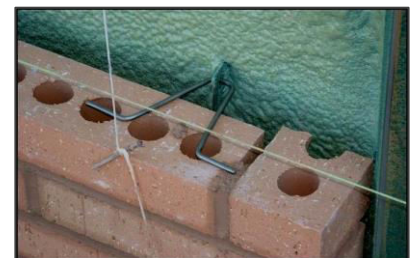
$S_p = (1.0)(1.0)(3.0) / 1.5 = 2.0$ (0.7 < S_p < 4.0)

This new formula in the 2006 BCBC may result in lower lateral seismic loads than the previous code version ($V_p = v \cdot I \cdot S_p \cdot W_p$, with $S_p = 5$).

This may result in wind loads governing in more cases.

Where $I_E F_a S_a(0.2) < 0.35$, these requirements do not apply for masonry veneer connections for buildings, other than post-disaster buildings.

The latest generation of strong, 2-piece adjustable ties can provide the opportunity for spacings up to the maximum allowable of 600 mm vertically and/or 820 mm (32") horizontally. For metric block and metric stud spacings, the effective horizontal maximum is 800 mm.



BRICK TIE DESIGN EXAMPLE

The structural design example below is based upon the requirements of the 2006 BCBC; CSA S304.1-04 Design of Masonry Structures; and CSA A370-04 Connectors for Masonry.

The process begins with the calculation of seismic and wind loads for a typical 2-storey school in Vancouver. The seismic case is assumed to govern for this example.

For this example, we try a tie with a factored resistance (design strength) of 1.1 kN. The allowable spacing of the ties may be affected by whether the back-up wall is concrete block or steel stud. The spacing for the non-flexible block wall is directly obtained by dividing the tie strength by the applied load in kN/m², which results in an area in this case of 0.83 m²

(8.9 sq. ft.) per tie. However, the maximum spacing for any tie system to metric block is limited to a maximum of 600 mm by 800 mm (.48 m²) (5.2 sq.ft.), so this spacing is specified. (actual horizontal spacing maximum is 820 mm to accommodate studs at 32 in.)

For the flexible steel stud back-up, additional requirements are applied to account for the less rigid support condition. For this case, a smaller wall area per tie of .42 m² (4.5 sq. ft.) is calculated, and the spacing options are based on stud space increments. The selection of a higher capacity tie may put both back-up systems into the maximum spacing condition.

PROJECT:

Vancouver, 2 storey school, on soft rock

Brick veneer on both concrete block, and steel stud back-up: stud spacing @ 400 mm, stud height 2.8 m.

1. LATERAL LOADS

- Wind Load: The Factored Wind Load calculation as determined for other components at the most severe location on the building may govern for veneer ties,

Staggered ties allowed:

A370-04 now allows ties to be placed in a staggered pattern. This may be more efficient, and ensure that all studs are loaded for wood and metal backups.

There must be a top row tie at every stud line.

compared to the seismic example below.

- Seismic Load: $V_p = 0.3 F_a S_a(0.2) I_E S_p W_p$
 $F_a = 1.0$; $S_a(0.2) = 0.94$; $I_E = 1.3$; $W_p = 1.8 \text{ kN/m}^2$
 $S_p = C_p A_r A_x / R_p = (1.0)(1.0)(3.0) / 1.5 = 2.0$

$$V_p = 0.3(1.0)(0.94)(1.3)(2.0)(1.8)$$
$$= 1.32 \text{ kN/m}^2$$

2. TIE SPACING (Assume Seismic governs for this example)

Examine spacing for a tie with a factored resistance of 1.1 kN (from manufacturers' test data literature)

- Concrete Block Back-up (non-flexible)
 - Area = $1.1 \text{ kN} / 1.32 \text{ kN/m}^2 = 0.83 \text{ m}^2 / \text{tie}$
 - Therefore space at the maximum 600 mm vertically by 800 mm horizontally. Area = 0.48 m^2
 - Could also consider lower capacity tie.
- Steel Stud Back-up (flexible)
 - Double tributary area load for flexible back-up:
Area = $1.1 \text{ kN} / (1.32 \text{ kN/m}^2 \times 2) = 0.42 \text{ m}^2 / \text{tie}$
 - Check 40% of stud load:
 $(40\%)(0.4\text{m})(2.8\text{m})(1.32) = 0.59 \text{ kN} / \text{tie}$
 $0.59 < 1.1 \text{ kN tie capacity} - \text{OK}$

Spacing options:

1. Maximum vertical spacing of 600 mm by the stud spacing of 400 mm horizontally. Area = 0.24 m^2 ($\leq 0.42 \text{ OK}$)
2. Horizontally at double stud spacing of 800 mm (staggered) by a vertical spacing of 450 mm. Area = 0.36 m^2 (≤ 0.42)
(Based on vertical brick module fitting 450 mm.)
3. Could also consider a higher capacity tie.

Note:

CSA A370-04 requires minimum unfactored tie strength of 1.0 kN.

Spacing at Openings and Tops and Bottoms of Walls:

- Openings: not more than 300mm from edge at maximum 600mm spacing.
- Top: not more than 300mm to top row, at every stud, even if staggered.
- Bottom: not more than 400mm to bottom row.